

**STATE OF VERMONT
AGENCY OF TRANSPORTATION**

**Scoping Report
FOR
Orwell STP Deck (41)**

TH 3 (Route 73), BRIDGE 4 OVER NORTH FORK CREEK

November 17, 2015



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I. Site Information

Bridge 4 is a town owned bridge located on Route 73 (TH 3) in District 3, Addison County, Orwell VT. The Bridge is located approximately 2 miles west of the junction with VT 22A.

Roadway Classification

Route 73	Major Collector
Bridge Type	Cast in Place Deck on Rolled Beam
Bridge Length	72 feet
Year Built	1946
Ownership	Town Owned

Need

The following is a list of deficiencies of Bridge 31:

1. The deck is rated a 5 (fair).
2. The width of the bridge is substandard.
3. The Bridge Rail including approach rail is substandard.

Traffic

Estimated AADT for 2015.

	Route 73	
TRAFFIC DATA	2016	2036
AADT	340	350
DHV	50	55
ADTT	25	35
%T	10.4	13.8
%D	53	53

Design Criteria

The design standards for this roadway are indicated below; for the situations that the Vermont State Standards do not apply.

1. AASHTO. *A Policy on Geometric Design of Highways and Streets*. Association of State Highway and Transportation Officials, Washington, DC, 2011. (The Green Book)
2. AASHTO. *Roadside Design Guide*. Association of State Highway and Transportation Officials, Washington, DC, 2011.
3. Minimum Standards are based on the Vermont State Design Standards:
http://vtransengineering.vermont.gov/sites/aot_program_development/files/documents/publications/VermontStateDesignStandards.pdf

Minimum standards are based on commentary from the Vermont State Design Standards for Lane and Shoulder widths for Urban Collectors.

Design Criteria	Source	Existing Condition	Minimum Standard	Comment
Bridge Lane and Shoulder Widths	Green Book Chapter 8.2	0'-10'-10'-0'	3'-11'-11'-3'	Substandard
Speed		40 mph (Posted)	40 mph (Design)	
Pedestrian Criteria		N/A	N/A	
Bridge Railing	Structures Design Manual Section 13	Concrete/Cable	TL-2	Substandard

Inspection Report Summary

Deck Rating 5 Serious
 Superstructure Rating 6 Satisfactory
 Substructure Rating 6 Satisfactory

06/30/15 Town has put temporary shoring in place along soffit where critical finding was last inspection. Center bay is saturated and there is still a potential for further problems. Washout along abutment 1 upstream side needs to be filled in. No changes in superstructure of substructure. MJK SP

6/17/14 Deck is in poor condition as deterioration is progressing along center bay. Full depth failure is highly possible. Large spall in soffit with rusted through rebar is present and 2nd layer of matting is exposed. Deck needs replacement in the meantime bay should be bunked with temporary shoring. Steel needs touch up painting and abutment 1 could use patching. Town letter Sent. MJK JM

6/27/12 Deck continues to deteriorate mainly along bay 2 where a deep spall is that has rusted through rebar and 1st layer of rebar matting along top is in view. Deck should be replaced along with updated guardrail system. ~ MJK JM

7/19/10 The deck continues to deteriorate. The bridge and approach guard rail should be upgraded to meet standards. DCP

Utilities

The existing utilities are as follows:

Municipal Utilities

- N/A

Public Utilities

Aerial:

- There are aerial electric and telephone facilities which run near the edge of the entire project area. These facilities may need to be relocated in order to facilitate the placement of the new bridge deck and or approach rail.

Overhead utilities may have to be relocated for construction.

Right Of Way

The existing Right-of-Way is plotted on the Layout Sheet. No additional Right-of-Way acquisition will be necessary.

Resources

Historic:

Bridge 4 is a historic structure.

Safety

The bridge is not in a High Crash location.

II. Alternatives Discussion

This Project was identified by Asset Management along with 10 other structures as a candidate to go through a Bridge Deck Pilot program. The objective of the program was to identify structures that could benefit from a bridge deck rehabilitation or replacement in order to extend the years of service of the bridge's superstructure and substructure. The scope is limited to the decks exclusively, therefore only three alternatives were evaluated as follows:

Alternative 1: No Action

This alternative would involve leaving the bridge in its current condition. The deck condition would require some additional maintenance or replacement within the next 10 years, therefore the No Action alternative is not recommended.

Alternative 2: Deck Patching

It would not be cost effective to try and salvage this deck. Given the level of deterioration the repairs required would need to be full depth. Additionally any repairs made would not improve the functionality or structural deficiencies of the bridge, therefore this alternative is not recommended.

Alternative 3: Deck Replacement

This alternative would involve removing the existing deck in its entirety and placing a new deck on the existing steel beams.

The existing substructure is in satisfactory condition, and it is reasonable to assume that it can safely carry anticipated traffic loads for an additional 40 years. No repairs would be recommended to the existing substructure at this time. However future projects may entail cleaning and patching, or cleaning and encasing the existing substructure.

Advantages: This alternative would address the structural deficiencies of the existing bridge, with minimum upfront costs. This option would have minimal impacts to adjacent properties and resources.

Maintenance of Traffic: Traffic could be maintained on an offsite detour or with phased construction. It generally does not make economic sense to construct a temporary bridge for a rehabilitation project.

Bridge Width

The existing bridge width is substandard. Given this is a maintenance project meeting new design standards may not be possible. However the scope of the project will be to improve the bridge width as much as possible given the site constraints. Additionally the current Bridge and Approach railing is substandard. The new railing will be a crash tested approved guardrail system.

III. Maintenance of Traffic

The Vermont Agency of Transportation reviews each new project to determine suitability for the Accelerated Bridge Program, which focuses on expedited delivery of construction plans, permitting, Right-of-Way, and faster construction of projects in the field. One practice that helps this endeavor is to close bridges for portions of the construction period, rather than provide temporary bridges. In addition to saving money, the intention is to minimize the closure period with faster construction techniques and incentives to contractors to complete projects sooner. The Agency will consider the closure option on most projects where rapid reconstruction or rehabilitation is feasible. The use of prefabricated elements in new bridges also expedites construction schedules. This can apply to decks, superstructures, and substructures. Accelerated bridge construction and short term road closures creates a safer working environment for construction personnel while minimizing traffic impacts. The following maintenance of traffic options have been considered:

Option 1: Off-Site Detour

Route 73:

This option would close the bridge and reroute traffic onto an offsite detour. Since the bridge is located on a Town Highway, it would be the responsibility of the Town of Orwell to choose, design and manage the preferred detour route and traffic control plan. The Town would also be responsible for management of emergency services throughout the closure period. A possible detour that may be considered by the Town is as follows:

1. East on Route 73 (TH 3), right onto Old Foundry Rd (TH 14), right onto Route 73 East (TH 3) for approximately 5.75 miles end to end.

A map of this detour route can be found in the Appendix.

Advantages: The costs associated with signing the detour are much lower than the construction costs associated with other maintenance of traffic options. By detouring traffic away from construction activities, it creates a safer working environment for the construction workers. By not constructing the structure in phases, there will be no vibrations or deflections from adjacent traffic to affect the quality of the closure pours joining the phases. By not requiring the construction and removal of temporary approaches, temporary bridges and temporary crossovers, the length of construction can be reduced over those other options. This is the safest traffic control option since the traveling public is removed from the construction site.

Disadvantages: Traffic will not be maintained along the existing corridor for a limited portion of construction. Through traffic will see an increase in travel times during the closure period.

Option 2: Phasing

Another method of maintaining traffic along the corridor during construction is to build a new structure one lane at a time, or in phases. This allows the road to stay open to traffic during construction, while having minimal impacts to adjacent property and environmental resources.

While the time required to design a phased construction project would remain the same, the onsite time required to complete a phased construction project increases because some of the construction tasks have to be performed multiple times. In addition to the increased design and construction costs mentioned above, the costs also increase for phased construction because of the inconvenience of working around traffic and the effort involved in coordinating the joints between the phases. Another negative aspect of phased construction is the decreased safety of the workers and vehicular traffic, which is caused by increasing the proximity and extending the duration that workers and moving vehicles are operating in the same confined space. Phased construction is usually considered when the benefits include reduced impacts to resources and decreased costs and development time by not requiring the purchase of additional ROW.

Advantages: Traffic would be maintained along the existing corridor during construction.

Disadvantages: While the time and cost required to construct a phased project may be less than that required to construct a project with a temporary bridge, the time required to construct a phased construction project is still longer than a project constructed without phasing, because some of the construction tasks have to be performed multiple times and cannot be performed concurrently. The costs of construction also increases over un-phased work because of this increase in the length of time, the additional inconvenience of working around traffic, and the effort involved in coordinating the joints between the phases. Once again, while the corridor will be open to traffic during construction, traffic will still be delayed and disrupted by the reduction in the number of lanes and by construction vehicles and equipment entering and exiting the site. The construction workers and equipment will still be in close proximity to vehicular traffic increasing the probability of accidents.

Option 3: Temporary Bridge

A temporary bridge was not considered given the additional costs associated with a temporary bridge. Such costs would make a rehabilitation project no longer cost effective.

Maintenance of Traffic Conclusion

Route 73:

Due to the availability and close proximity of local roads to detour traffic phasing will not be considered further in this report. Phasing would be more expensive, take longer to construct, and produce a lower quality final product at the completion of construction. Thus an Off-Site Detour would be the recommended maintenance of traffic option at this location and will only be considered in the cost matrix.

IV. Cost Matrix¹

Orwell STP Deck (41)		Alt 1 Do Nothing	Alt 3 Deck Replacement
			<i>a. Conventional</i>
COST	Bridge Cost	\$0	\$173,000
	Removal of Structure	\$0	\$65,000
	Roadway (Includes Mobilization)	\$0	\$153,000
	Traffic Control	\$0	\$24,000
	Construction Costs	\$0	\$415,000
	Construction Engineering + Contingencies	\$0	\$124,500
	Total Construction Costs w CEC	\$0	\$539,500
	Preliminary Engineering²	\$0	\$84,000
	Right of Way	\$0	\$0
	Total Project Costs	\$0	\$623,500
	Annualized Costs	\$0	\$0
TOWN SHARE	Towns total Share (2.5%)		\$15,587.50
SCHEDULING	Project Development Duration ³		1 years
	Construction Duration		1 years
	Closure Duration (If Applicable)		60 Days
ENGINEERING	Typical Section –		
	Typical Section –		
	Typical Section – Bridge (feet)	4'-10'-10'-4" (20'-8")	6'-10'-10'-6" (21')
	Geometric Design Criteria	No Change	No Change
	Traffic Safety	No Change	No Change
	Alignment Change	No Change	No Change
	Bicycle Access	No Change	No Change
	Vertical Clearance	No Change	No Change
	Pedestrian Access	No Change	No Change
	Utility	No Change	Relocation Possible
OTHER	ROW Acquisition	No	No
	Road Closure	No	Yes
	Design Life	<5 years	40 years

¹ Costs are estimates only, used for comparison purposes.

² Preliminary Engineering costs are estimated starting from the end of the Project Definition Phase.

³ Project Development Durations are starting from the end of the Project Definition Phase.

V. Conclusion

We recommend **Alternative 3**; to replace the existing deck using a road closure.

Structure:

The recommended alternative includes replacing the existing deck with a cast-in-place deck using conventional construction methods. The new structure will feature a composite concrete deck which allows the bridge to be widened slightly to improve the typical section. Crash tested approach and bridge rail will also be provided as part of this project which will be an improvement upon the current guardrail system. The proposed improvements should extend the life expectancy of this structure an additional 40 years.

Traffic Maintenance:

It is recommended that traffic be maintained on an offsite detour. There are several reasonable detour routes that could be signed by the Town of Orwell. Therefore, it is reasonable to close the road and reroute traffic while the new bridge deck is constructed. By not providing a temporary bridge, both the project development time and the project cost are significantly reduced. Additionally, in accordance with Act 153, by closing the bridge to traffic during construction, the local share is reduced by 50% from a 5% town share to a share of only 2.5% of the project costs.

VI. Appendices

- Site Pictures
- Town Map
- Detour
- Bridge Inspection Report

Site Pictures





Detour/Local Bypass

Driving Directions from 173 Route 73, Orwell, Vermont 05760 to Hemenway Hill Rd, Or... Page 2 of 2

Total Travel Estimate: 5.24 miles - about 11 minutes



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Bridge Inspection Report

STRUCTURE INSPECTION, INVENTORY and APPRAISAL SHEET Vermont Agency of Transportation – Structures Section – Bridge Management and Inspection Unit		
Inspection Report for ORWELL	bridge no.: 00004	District: 3
Located on: TR 03 FAS 156 ove NORTH FORK CREEK	approximately 2.0 MI W JCT VT 22A	Owner: 03 TOWN-OWNED
CONDITION Deck Rating: 5 FAIR Superstructure Rating: 6 SATISFACTORY Substructure Rating: 6 SATISFACTORY Channel Rating: 8 VERY GOOD Culvert Rating: N NOT APPLICABLE Federal Str. Number: 200156000401142 Federal Sufficiency Rating: 054.8 Deficiency Status of Structure: SD	STRUCTURE TYPE and MATERIALS Bridge Type: ROLLED BEAM Number of Approach Spans: 0000 Number of Main Spans: 001 Kind of Material and/or Design: 3 STEEL Deck Structure Type: 1 CONCRETE CIP Type of Wearing Surface: 6 BITUMINOUS Type of Membrane: 2 PREFORMED FABRIC Deck Protection: 0 NONE	APPRaisal *AS COMPARED TO FEDERAL STANDARDS Bridge Railings: 0 DOES NOT MEET CURRENT STANDARD Transitions: 0 DOES NOT MEET CURRENT STANDARD Approach Guardrail: 0 DOES NOT MEET CURRENT STANDARD Approach Guardrail Ends: 0 DOES NOT MEET CURRENT STANDARD Structural Evaluation: 5 BETTER THAN MINIMUM TOLERABLE CRITERIA Deck Geometry: 4 MEETS MINIMUM TOLERABLE CRITERIA Underclearances Vertical and Horizontal: N NOT APPLICABLE Waterway Adequacy: 8 SLIGHT CHANCE OF OVERTOPPING ROADWAY Approach Roadway Alignment: 8 EQUAL TO DESIRABLE CRITERIA Scour Critical Bridges: 8 STABLE FOR SCOUR
AGE and SERVICE Year Built: 1946 Year Reconstructed: 0000 Service On: 1 HIGHWAY Service Under: 5 WATERWAY Lanes On the Structure: 02 Lanes Under the Structure: 00 Bypass, Detour Length (miles): 00 ADT: 000340 % Truck ADT: 06 Year of ADT: 1995	GEOMETRIC DATA Length of Maximum Span (ft): 0072 Structure Length (ft): 000074 Lt Curb/Sidewalk Width (ft): 0.4 Rt Curb/Sidewalk Width (ft): 0.4 Bridge Rdwy Width Curb-to-Curb (ft): 20.8 Deck Width Out-to-Out (ft): 22.5 Appr. Roadway Width (ft): 020 Skew: 00 Bridge Median: 0 NO MEDIAN Min Vertical Clr Over (ft): 99 FT 99 IN Feature Under: FEATURE NOT A HIGHWAY OR RAILROAD Min Vertical Underclr (ft): 00 FT 00 IN	DESIGN VEHICLE, RATING, and POSTING Load Rating Method (Inv): 1 LOAD FACTOR (LF) Posting Status: A OPEN, NO RESTRICTION Bridge Posting: 5 NO POSTING REQUIRED Load Posting: 10 NO LOAD POSTING SIGNS ARE NEEDED Posted Vehicle: POSTING NOT REQUIRED Posted Weight (tons): Design Load: 2 H 15
INSPECTION SUMMARY and NEEDS 06/30/15 Town has put temporary shoring in place along soffit where critical finding was last inspection. Center bay is saturated and there is still a potential for further problems. Washout along abutment 1 upstream side needs to be filled in. No changes in superstructure of substructure. MJK SP 6/17/14 Deck is in poor condition as deterioration is progressing along center bay. Full depth failure is highly possible. Large spall in soffit with rusted through rebar is present and 2nd layer of matting is exposed. Deck needs replacement in the meantime bay should be bunked with temporary shoring. Steel needs touch up painting & abutment 1 could use patching. Town letter sent. MJK JM 06/27/12 Deck continues to deteriorate mainly along bay 2 where a deep spall is that has rusted through rebar and 1st layer of rebar matting along top is in view. Deck should be replaced along with updated guardrail system. -MJK JM The deck continues to deteriorate. The bridge and approach guard rail should be up graded to meet standards. 7/19/10 DCP	INSPECTION and CROSS REFERENCE X-Ref. Route: Insp. Date: 062015 Insp. Freq. (months) 24 X-Ref. BrNum:	

Friday, July 10, 2015